



Buckling and Post-buckling of Plates, Fig. 1 A discrete Elastica. The configuration shown is not an equilibrium: buckling can occur only when the applied force is opposite the x -axis ($\underline{f} < 0$)

Note that we use an underline for all vectors. This leads to a buckling problem, and we are particularly interested in equilibrium configurations of the system that break symmetry, such that spring is no longer aligned with the x -axis.

An Elastic Spring Resting on an Elastic Hinge

The fixed position of the hinge is used as the origin of coordinates. Let us denote by $\underline{r} = (x, y)$ the position of the other end of the spring, where the force is applied. The elongation of the spring and the rotation at the hinge are measured by a stretching strain $\varepsilon(\underline{r})$ and a curvature strain $\kappa(\underline{r})$, respectively,

$$\begin{aligned}\varepsilon(\underline{r}) &= \sqrt{x^2 + y^2} - \ell^* \\ \kappa(\underline{r}) &= \tan^{-1}(y/x),\end{aligned}$$

where ℓ^* is the reference length of the spring. Note that we refer to ε and κ as a stretching and curvature strain, respectively, but they actually are a length and an angle, respectively: we do so to keep consistency with the plate model to be introduced later on.

The total potential energy \mathcal{E}_{dE} of the discrete Elastica reads

$$\mathcal{E}_{\text{dE}}(\underline{r}) = w(\varepsilon(\underline{r}), \eta^2 \kappa^2(\underline{r})) - \eta^2 \underline{f} \cdot \underline{r}, \quad (1)$$

where $w(\varepsilon, \eta^2 \kappa^2)$ is the total elastic energy stored in the spring and in the hinge and $-\eta^2 \underline{f} \cdot \underline{r}$ is the potential energy associated with the external force, which we assume to be dead (non-follower) for the moment.

The strain energy w has been assumed to depend on the bending strain through $(\eta\kappa)^2$ for the following reasons. First, the square in $(\eta\kappa)^2$ reflects the mirror symmetry $(y, \kappa) \leftarrow (-y, -\kappa)$ of the system, which warrants that the strain energy is an even function of κ , i.e., a smooth function of κ^2 . Second, κ appears multiplied by η , which is a small parameter intended to play the role of the aspect ratio h/L of a plate,

$$\eta \rightarrow 0.$$

The effect of making the strain energy a function of $(\eta\kappa)^2$ rather than simply κ^2 is that the bending modulus (i.e., the elastic modulus of the hinge) will scale like η^2 times the modulus of the spring. This is indeed the case in elastic plates, and we need to retain this key feature of the plate theory.

Let $\underline{r}_0 = (\ell^*, 0)$ describe the natural configuration of the system. By definition this is an equilibrium in the absence of external load, $\underline{f} = \underline{0}$, which leads to the requirement

$$w_{,\varepsilon}(0, 0) = 0. \quad (2)$$

Note that the external force \underline{f} has been rescaled by η^2 in (1), as this will allow the critical load to scale consistently. This will be further explained below.

Here $w_{,\varepsilon}$ denotes the derivative of w with respect to its first argument.

The form of the strain energy w we use is completely general. One could assume that it splits into two contributions $w(\varepsilon, \eta^2 \kappa^2) = e_b(\eta^2 \kappa^2) + e_s(\varepsilon(\underline{r}))$, where e_b is the bending energy e_b stored in the elastic hinge and e_s the stretching energy stored in the spring. We will not assume that this decomposition holds, however: we want to use a model that makes as few assumptions as possible, with the aim to identify all the features of the model that influence the results of the stability analysis. Similarly, one could consider linearly

Buckling And Postbuckling Of Composite Plates

G.J. Turvey, I.H. Marshall



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Mechanics of Composite Structures Christian Mittelstedt, 2025-06-06 This book includes a detailed introduction to the mechanics of composite structures i e thin walled laminated structures as they are frequently used in lightweight construction and design The book is subdivided into five parts Starting from the basics of linear elasticity in part I part II is devoted to the mechanics of composite laminates as relevant for the further course of this book and as employed in industrial practice Part III deals with the buckling and postbuckling behaviour of unstiffened and stiffened thin walled laminated structures and presents closed form analytical and semi analytical approaches for the stability analysis of such structures Part IV includes some advanced topics such as free edge effects and laminated shells before part V concludes with the analysis of laminated beam structures This book is intended for students at universities but also for engineers in practice and researchers in engineering science

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